

# PROPOSED DEVELOPMENT AT

## 48 Cornelia St, Wiley Park

### GENERAL

- G1. STRUCTURAL DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE SPECIFICATION, ARCHITECTURAL, CIVIL & RELEVANT ENGINEERING SERVICES DOCUMENTS AND WITH OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED.
- G2. ALL DIMENSIONS SHOWN SHALL BE VERIFIED ON SITE. ENGINEERS DRAWINGS MUST NOT BE SCALED.
- G3. DURING CONSTRUCTION THE STRUCTURE SHALL BE MAINTAINED IN A STABLE CONDITION & NO PART SHALL BE OVERSTRESSED.
- G4. ALL MATERIALS & WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS.
- G5. UNLESS OTHERWISE NOTED ALL LEVELS ARE IN METERS & ALL DIMENSIONS ARE IN MILLIMETERS.
- G6. THESE DRAWINGS ARE SIGNED SUBJECT TO A CERTIFICATE OF INSPECTION BEING ISSUED BY THIS OFFICE. ALL REINFORCEMENT SHALL BE INSPECTED BY THIS OFFICE PRIOR TO PLACING CONCRETE.
- G7. BRITTLE FLOOR COVERING SUCH AS CERAMIC TILES SHOULD BE LAID USING AN APPROVED FLEXIBLE ADHESIVE SYSTEM TO CONTROL THE EFFECT OF SHRINKAGE CRACKING. A MINIMUM PERIOD OF THREE MONTHS DRYING OF THE CONCRETE IS USUALLY REQUIRED BEFORE THE PLACEMENT OF BRITTLE FLOOR COVERING.
- G8. SUBTERRANEAN TERMITE PROTECTION IS TO BE PROVIDED IN ACCORDANCE WITH AS 3660.1 WITH ALL JOINTS ADEQUATELY LAPPED AND TAPED AT PENETRATIONS.

### FOOTINGS

- F1. REQUIRED MINIMUM SAFE BEARING CAPACITY OF MATERIAL SHALL BE 150 KPa FOR SHALLOW FOOTINGS & 700 KPa FOR PILING; TO BE CONFIRMED BY GEOTECHNICAL ENGINEER.
- F2. SITE CLASS IN ACCORDANCE TO AS2870 IS : M
- F3. ALL WALLS AND COLUMNS SHALL BE CONCENTRIC WITH SUPPORTING FOOTING U.N.O.
- F4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING EXCAVATIONS IN A STABLE CONDITION WITHOUT AFFECTING ADJACENT PROPERTIES OR SERVICES. WHERE REQUIRED, TEMPORARY SHORING SHALL BE PROVIDED TO THE SIDES OF FOOTING EXCAVATION.
- F5. EXCAVATION SHALL NOT EXTEND BELOW A LINE DIPPING AT 45° FOR CLAY AND 30° FOR SAND AND AWAY FROM THE NEAREST UNDERSIDE CORNER OF ANY EXISTING FOOTINGS.

### SUBGRADE PREPARATION

- SP1. THE SITE SHALL BE EXCAVATED TO THE LEVELS SHOWN ON THE RELEVANT DRAWINGS.
- SP2. ALL TOPSOIL, ORGANIC AND DELETERIOUS MATERIAL IS TO BE STRIPPED FROM THE BUILDING SITE.
- SP3. SELECTED FILLINGS/HARD-CORE ETC. & SAND BLINDING UNDER SLABS SHOWN ON DRAWINGS SHALL BE PLACED IN LOOSE LAYERS NOT EXCEEDING 150mm & COMPACTED TO 98% OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS 1289 E1.1 (DENOTED AS STRUCTURAL FILLING).
- SP4. THE OWNERS ATTENTION SHOULD BE DRAWN TO APPENDIX B OF AS 2870 "PERFORMANCE REQUIREMENTS AND FOUNDATIONS MAINTENANCE" ON COMPLETION OF THE JOB.
- SP6. FILL MATERIAL BENEATH SLAB IS TO BE COMPACTED IN ACCORDANCE WITH AS 2870 & THE GEOTECHNICAL REPORT.
- SP7. THE SLAB IS TO BE ENTIRELY UNDERLAID WITH A 0.2mm POLYETHYLENE VAPOUR BARRIER WITH ALL JOINTS ADEQUATELY LAPPED AND TAPED AT PENETRATION.

### LOADINGS

- L1. Importance Level of Building: 2
- L2. Superimposed floor live loads are generally in accordance with AS/NZS 1170.1 and specifically:
- 1.5 KPa. GENERALLY
- 2.0 KPa. BALCONIES AND STAIRS
- L3. Wind loads have been determined in accordance with AS4055
- Wind Region: A Terrain Category: 3
- Topographic Class: 2 Shielding: PS
- Wind Classification: N2
- L4. The relevant provisions of AS1170.4 have been applied for the following Earthquake Design parameters:
- Probability factor Kp: 1 Hazard factor Z: 0.08
- Site Sub-Soil Class: Ce
- Earthquake Design Category: I

### CONCRETE

- C1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS3600.
- C2. ALL CONCRETE TO BE IN ACCORDANCE WITH AS 1379.
- C3. CONCRETE TO BE NORMAL CLASS CONCRETE WITH STRENGTH GRADE AND COVER TO REINFORCEMENT FOR PARTICULAR ELEMENTS AS FOLLOWS UNLESS NOTED OTHERWISE.

ELEMENT	GRADE DESIGNATION	COVER
RESIDENTIAL FOOTINGS	N25	50mm
STRIP/PAD FOOTINGS	N32	50mm
COLUMNS	N40	35mm
RC WALLS	N40	30mm
SLAB ON GROUND		
EXTERNAL	N25	40mm
INTERNAL	N25	30mm
SUSPENDED SLABS		
EXTERNAL	N32	40mm
INTERNAL	N32	30mm

- C4. WHERE A SLAB THICKNESS IS NOTED ON PLANS, THE FOLLOWING REQUIREMENTS MUST BE MET:
- MAXIMUM SLAB THICKNESS INCREASE OF 25% FROM NOTED MINIMUM VALUE.
  - LAY ALL REINFORCEMENT TO FALLS UNLESS NOTED OTHERWISE MAINTAINING MAXIMUM TOP COVER TO REINFORCEMENT OF 40mm. PROVIDE ADDITIONAL SL82 MESH IF COVER EXCEEDS 40mm.
  - PROPOSED CHANGES TO DESIGN INTENT TO BE SUBMITTED TO ENGINEER PRIOR TO CONSTRUCTION.
- C5. ALL NORMAL CLASS CONCRETE TO HAVE THE FOLLOWING BASIC PARAMETERS U.N.O.:
- SLUMP AT POINT OF ACCEPTANCE - MAIN CONTRACTOR TO SELECT SLUMP TO SUIT CONSTRUCTION REQUIREMENT AS ONE OF 70mm, 80mm, 90mm, 110mm, OR 120mm.
  - MAXIMUM NOMINAL SIZE OF AGGREGATE - 20mm.
  - SUPPLIER TO SAMPLE TEST AND ASSESS CONCRETE COMPLIANCE IN ACCORDANCE WITH PROJECT ASSESSMENT OF STRENGTH GRADE.
  - NO AIR ENTRAINMENT.
- C6. ALL CONCRETE SURFACES ARE TO BE CURED FOR SEVEN DAYS AFTER CASTING USING A PROPRIETARY LIQUID MEMBRANE CURING COMPOUND THAT CONFORMS TO AS 3799 AND IS COMPATIBLE WITH THE SPECIFIED FINISHES. CURING TO COMMENCE AS SOON AS PRACTICABLE BUT NOT MORE THAN 3 HOURS AFTER THE COMPLETION OF THE FINISHING OPERATIONS. THE SIDES OF SLABS/ BEAMS/ PANELS EXPOSED BY THE REMOVAL OF FORMS SHALL BE CURED IN ACCORDANCE WITH THE ABOVE.
- C8. CONDUITS IN SLABS TO BE MAXIMUM 25mm DIAMETER, LOCATED IN CENTRE OF SLAB, WITH MINIMUM 500 SPACING BETWEEN CONDUITS. IF LARGER AND/OR MORE CONDUITS REQUIRED, CONTACT ENGINEER WITH DETAILS PRIOR TO CONSTRUCTION FOR POSSIBLE RE-DESIGN AND APPROVAL OF REVISED ALLOWABLE CONDUITS.
- C9. FROM CONSTRUCTION JOINTS ONLY WHERE APPROVED BY THE ENGINEER PRIOR TO CONSTRUCTION CONTACT ENGINEER FOR REVISING ANY LOCATIONS TO SUIT CONSTRUCTIONS REQUIREMENTS.

### REINFORCEMENT

- R1. ALL REINFORCING TO BE IN ACCORDANCE WITH AS 4671.
- R2. ALL REINFORCEMENT BARS AND MESH TO BE DEFORMED AND STRENGTH GRADE 500.
- R3. CHAIR UP ALL REINFORCING AT 800mm MAXIMUM CENTRES TO ENSURE CORRECT COVER DURING CONCRETE POUR. PLASTIC CHAIRS TO BE USED IN EXTERNAL SLABS.
- R4. REINFORCEMENT IS SHOWN DIAGRAMMATICALLY AND NOT NECESSARILY IN TRUE PROJECTION
- R5. NOTIFY THE ENGINEER A MINIMUM OF 24 HOURS BEFORE REINFORCEMENT HAS BEEN COMPLETED. ALLOW 2 HOURS AFTER THE COMPLETION OF THE REINFORCEMENT FOR THE ENGINEER'S INSPECTION. DO NOT ORDER CONCRETE UNTIL REINFORCEMENT HAS BEEN APPROVED BY THE ENGINEE

REINFORCEMENT		
BAR SIZE	HORIZONTAL LAP LENGTH	VERTICAL LAP LENGTH
N12	500	500
N16	700	650
N20	950	800
N24	1250	1000
N28	1550	1150
N32	1850	1300
N36	2200	1500

### BRICKWORK AND BLOCKWORK

- B1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE CURRENT SAA MASONRY CODE, AS 3700 EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.
- B2. ALL LOAD BEARING BRICKS SHALL BE LAID FROGS UP EXCEPT FOR THE TOP COURSE, WHICH SHALL BE LAID FROGS DOWN. WHEN SUPPORTING A CONCRETE SLAB OR BEAM BRICKWORK SHALL HAVE A LAYER OF MORTAR PLACED ON THE TOP AND TROWELLED SMOOTH, THE TOP 2 COURSES OF BRICKS SHALL BE LAID WITH REINFORCEMENT IN THE JOINTS.
- B3. WHERE WALLS ARE NON LOAD BEARING AT EITHER HORIZONTAL OR VERTICAL FACES THEY SHALL BE SEPARATED FROM THE CONCRETE BY 20mm THICK 'CANEITE' OR EXPANDED POLYSTYRENE U.N.O.
- B4. NO HOLES OR CHASES SHALL BE CUT IN LOAD BEARING BRICKWORK OR BLOCKWORK WITHOUT THE PRIOR APPROVAL OF THE ENGINEER.
- B5. PROVIDE ALL BRICKS OF STRENGTH f<sub>uc</sub> = 20 MPa U.N.O.
- B6. PROVIDE ALL HOLLOW CONCRETE MASONRY OF STRENGTH f<sub>uc</sub> = 15 MPa U.N.O.
- B7. PROVIDE CLASSIFICATION M3 MASONRY MORTAR U.N.O. NOTE THAT WITHIN 100m FROM NON-SURF COAST, OR 1Km SURF COAST, PROVIDE CLASSIFICATION M4 MORTAR.
- B8. CORES TO BE FILLED WHERE REQUIRED WITH CONCRETE OF STRENGTH f<sub>c</sub> = 20 MPa, 10mm MAX. AGGREGATE SIZE AND A MAX. SLUMP OF 180.
- B9. DO NOT ERECT MASONRY SUPPORTED BY CONCRETE SLABS OR BEAMS UNTIL ALL FORMWORK AND PROPS UNDER HAVE BEEN REMOVED.
- B10. PROVIDE MOVEMENT CONTROL JOINTS VERTICALLY FOR FULL HEIGHT OF WALL AS FOLLOWS:
- FOR GENERAL MASONRY = 8m MAXIMUM CENTERS & 4m MAXIMUM FROM CORNERS.
- FOR ARTICULATED MASONRY = 6m MAXIMUM CENTERS & 4m MAXIMUM FROM CORNERS.
- PROVIDE 15mm MINIMUM JOINTS WITH AN APPROVED COMPRESSIBLE FILLER, TIED TOGETHER EVERY 4TH COURSE WITH AN MET 3.3 MASONRY SLIDING TIE OR APPROVED EQUAL.

### TIMBER NOTES

- T1. ALL TIMBER DESIGN AND CONSTRUCTION TO BE AS1720 U.N.O.
- T2. AS 1684 IS RELEVANT TO DOMESTIC CONSTRUCTION IN SHELTERED LOCATIONS.
- T3. SOFTWOOD MINIMUM GRADE F7 U.N.O. HARDWOOD MINIMUM GRADE F11 U.N.O.
- T4. EXTERNAL TIMBER TO BE EITHER HARDWOOD DURABILITY CLASS I OR II OR IMPREGNATED GRADE F7. PRESSURE TREATED TO AS 1684 AND RE-DRILLED PRIOR TO USE. SUPPLEMENTARY TREATMENT SHALL BE APPLIED TO ALL CUT SURFACES.
- T5. ALL BOLTS IN TIMBER CONSTRUCTION TO BE MIN. M16 U.N.O. BOLT HOLES TO BE DRILLED EXACT SIZE. WASHERS UNDER HEADS AND NUTS TO BE AT LEAST 2.5 TIMES BOLT DIAMETER.
- T6. ALL TIMBER JOINTS AND NOTCHES TO BE 100mm MINIMUM FROM LOOSE KNOTS. SEVERE SLOPING GRAIN, GUM VEINS OR OTHER MINOR DEFECTS.
- T7. BLOCKING IS NOT REQUIRED FOR JOISTS SPANNING LESS THAN 3m. FOR JOISTS SPANNING GREATER THAN 3m AND LESS THAN 4.2m PROVIDE ONE ROW OF BLOCKING MID-SPAN. FOR JOISTS SPANNING GREATER THAN 4.2m AND UP TO 6.0m PROVIDE TWO ROWS OF BLOCKING AT  $\frac{1}{3}$  POINTS. FOR DEEP JOISTED FLOORS WHERE A CONTINUOUS TRIMMING JOIST IS NOT PROVIDED AT END OF JOISTS, BLOCKING IS REQUIRED AT 1800mm MAXIMUM CENTERS. (REFER TO AS 1684)

### STRUCTURAL STEEL

- S1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS 1163, AS 1594, AS 3678, AS 3679, AS 4100 AND AS 4600 AS APPROPRIATE.
- S2. REFER TO SPECIFICATION FOR COATING OF STEELWORK. WHERE NO SPECIFICATION EXISTS, THE MINIMUM PRIMING TREATMENT SHALL BE AS FOLLOWS:
- CLEAN STEELWORK FREE OF ALL RUST, MILLSCALE, OIL, GREASE AND DELETERIOUS MATERIAL USING ABRASIVE BLAST CLEANING.
  - MIN. SURFACE PREPARATION IN ACCORDANCE WITH AS 1627 SHALL BE CLASS 2 1/2.
  - WITHIN 4 HOURS OF PREPARATION, SHOP PRIME WITH SINGLE COAT (MINIMUM 75 MICRON) OF HIGH BUILD ZINC PHOSPHATE (COLOUR AS SPECIFIED BY ARCHITECT).

REFER TO ARCHITECT FOR ANY TOP COAT REQUIREMENTS.

- S3. IN ACCORDANCE WITH AS 4680 HOT DIP GALVANIZE THE FOLLOWING ITEMS:
- EXTERNALLY EXPOSED STEELWORK (MINIMUM ZINC COATING THICKNESS TO BE 600g/m<sup>2</sup>).
- S4. IT IS NOT PERMISSIBLE TO SUBSTITUTE DURAGAL SECTIONS FOR HOT DIP GALVANIZED SECTIONS WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER.
- S5. PROVIDE DRAINAGE HOLES FOR HOT DIP WHERE EXPOSED.
- S6. CHECK ALL DIMENSIONS ON SITE PRIOR TO FABRICATION OF STEELWORK.
- S7. STEEL GRADES AS PER AS 3679 TO BE AS FOLLOWS UNLESS NOTED OTHERWISE:
- |                         |                 |
|-------------------------|-----------------|
| • HOT ROLLED SECTIONS:  | GRADES 300 PLUS |
| • WELDED SECTIONS:      | GRADES 300      |
| • ANGLE SECTIONS:       | GRADE 300 PLUS  |
| • RHS AND SHS SECTIONS: | GRADE C350      |
| • CHS SECTIONS:         |                 |
| UP TO 165.1 DIA         | GRADE C250      |
| OVER 165.1 DIA          | GRADE 250       |
| • FLATS AND PLATES:     | GRADE 250       |
| • PURLINS AND GIRTS:    | GRADE 450       |
- S8. ALL PLATES TO BE 10mm PLATE UNLESS NOTES OTHERWISE.
- S9. ALL BOLTS BETWEEN STEEL MEMBERS TO BE M20 8.8/S UNLESS NOTES OTHER WISE.
- S10. ALL EXTERIOR BOLTS, NUTS AND WASHERS TO BE HOT DIP GALVANIZED.
- S11. THE NOMINAL DIAMETER OF A COMPLETED HOLE OTHER THAN A HOLE IN A BASE PLATE SHALL BE 2mm LARGER THAN THE NOMINAL BOLT DIAMETER FOR A BOLT NOT EXCEEDING 24mm IN DIAMETER AND NOT MORE THAN 3mm LARGER FOR A BOLT OF A GREATER DIAMETER.
- S12. FOR A HOLE IN A BASE PLATE, THE HOLE DIAMETER SHALL BE NOT MORE THAN 6mm GREATER THAN THE ANCHOR BOLT DIAMETER. A SPECIAL PLATE WASHER OF A MINIMUM THICKNESS 4mm SHALL BE USED UNDER NUT IF THE HOLE DIAMETER IS 3mm OR MORE LARGER THAN THE BOLTS DIAMETER. THE PLATE WASHER SHALL COMPLETELY COVER THE HOLE SUCH THAT THE MINIMUM DISTANCE FROM THE EDGE OF THE HOLE TO THE EDGE OF THE PLATE WASHER SHALL BE 0.5 TIMES THE HOLE DIAMETER.
- S13. ALL WELDS TO BE AS FOLLOWS UNLESS NOTES OTHERWISE:
- STRUCTURAL PURPOSE (SP).
  - 6mm CONTINUOUS FILLET WELD (CFW) (3mm MIG FOR LIGHT GAUGE SECTIONS).
  - WELD ELECTRODES FOR FILLET WELDS TO BE E48XX/ W50X.
- S14. FABRICATOR IS TO PROVIDE THE ENGINEER WITH 3 COPIES OF WORKSHOP DRAWINGS FOR INSPECTION BEFORE COMMENCEMENT OF FABRICATION. FABRICATOR IS TO ALLOW 10 WORKING DAYS FOR REVIEW OF DRAWINGS.
- S15. DO NOT MAKE ANY PENETRATIONS OR CUTS OTHER THAN THOSE SHOWN IN THE DRAWINGS WITHOUT PRIOR APPROVAL OF THE ENGINEER.
- S16. WELDING OF GALVANISED STEEL TO BE AVOIDED. WHERE UNAVOIDABLE, CORROSION PROTECTION SYSTEM TO BE APPLIED THAT PROVIDED EQUIVALENT PROTECTION AND DESIGN LIFE AS A HOT DIP GALVANISING WITH 600g/m<sup>2</sup> ZINC COATING.
- S17. PURLINS TO BE LAPPED MIN 15% OF ADJACENT SPAN UNLESS NOTED OTHERWISE.
- S18. UNLESS NOTED OTHERWISE, BOLTS AT PURLIN CLEATS AND LAPS TO BE:
- FOR PURLINS UP TO 250mm DEEP, 2-M12 4.6/S PURLIN BOLTS.
  - FOR PURLINS GREATER THAN 250mm DEEP, 2-M16 4.6/S PURLIN BOLTS.
- S19. UNLESS NOTED OTHERWISE, EACH PURLIN LAP IS TO HAVE TWO BOLTS AS ABOVE, WITH ONE BOLT THROUGH TOP WEB HOLE AND ONE BOLT THROUGH BOTTOM FLANGE. TWO BOLTS IN WEB IS NOT PERMITTED.

### CONSULTANTS:

### REV DATE DESCRIPTION

1	28/01/22	

### COUNCIL: DRAWN BY: DESIGNED BY: CLIENT:

Canterbury-Bankstown Council  
A.H  
R.D  
B. Adasi

### DRAWING TITLE:

Structural Notes

### SITE ADDRESS: 48 Cornelia St, Wiley Park 2195

LOT: - | SEC: - | SP: 20535

ISSUED FOR: CDC

PROJECT: Addition & Alteration

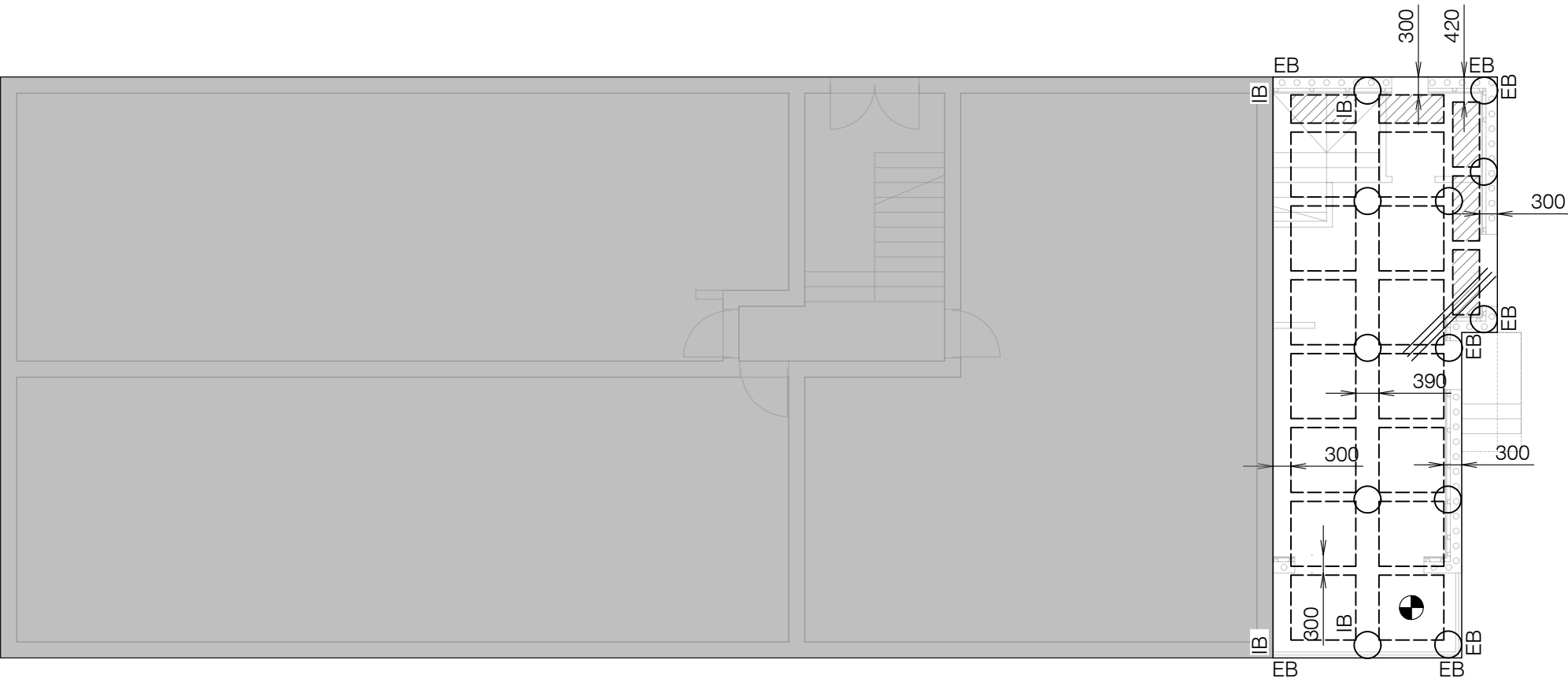
SCALE: N.T.S

DATE: 28/01/22

REV: R1

SHEET NO.: S-00

PROJECT NO 22-004



GROUND FLOOR SLAB PLAN

- SCALE 1:100
- NOTES:
- 1. 1. DRAWINGS TO BE READ IN CONJUNCTION WITH ARCHITECTURALS.
  - 2. REFER TO ARCHITECTURAL DRAWINGS FOR ALL SETOUT, LEVELS, FALLS ETC.
  - 3. DURING CONSTRUCTION, FOUNDATION MATERIAL HAS TO BE CHECKED AND VERIFIED BY GEOTECHNICAL ENGINEER TO ENSURE THE 100 KPA BEARING CAPACITY IS REACHED AS DESIGNED FOR.
  - 4. SLAB ON GROUND IS 120mm U.N.O WITH SL82 MESH TOP
  - 5. DETAILS AND REINFORCEMENTS TO BE CHECKED BY A STRUCTURAL ENGINEER PRIOR TO THE POURING OF CONCRETE

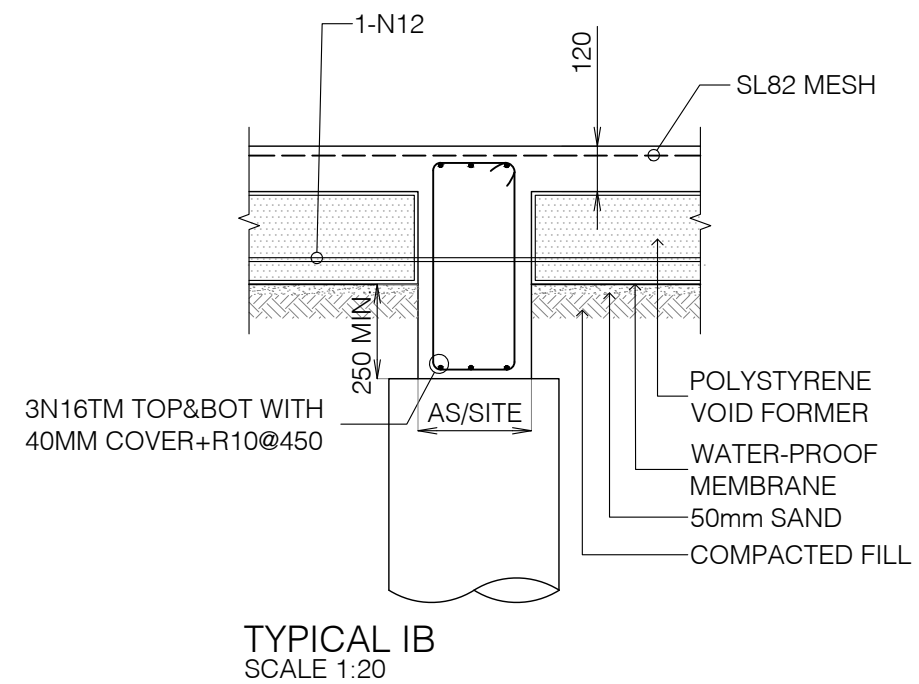
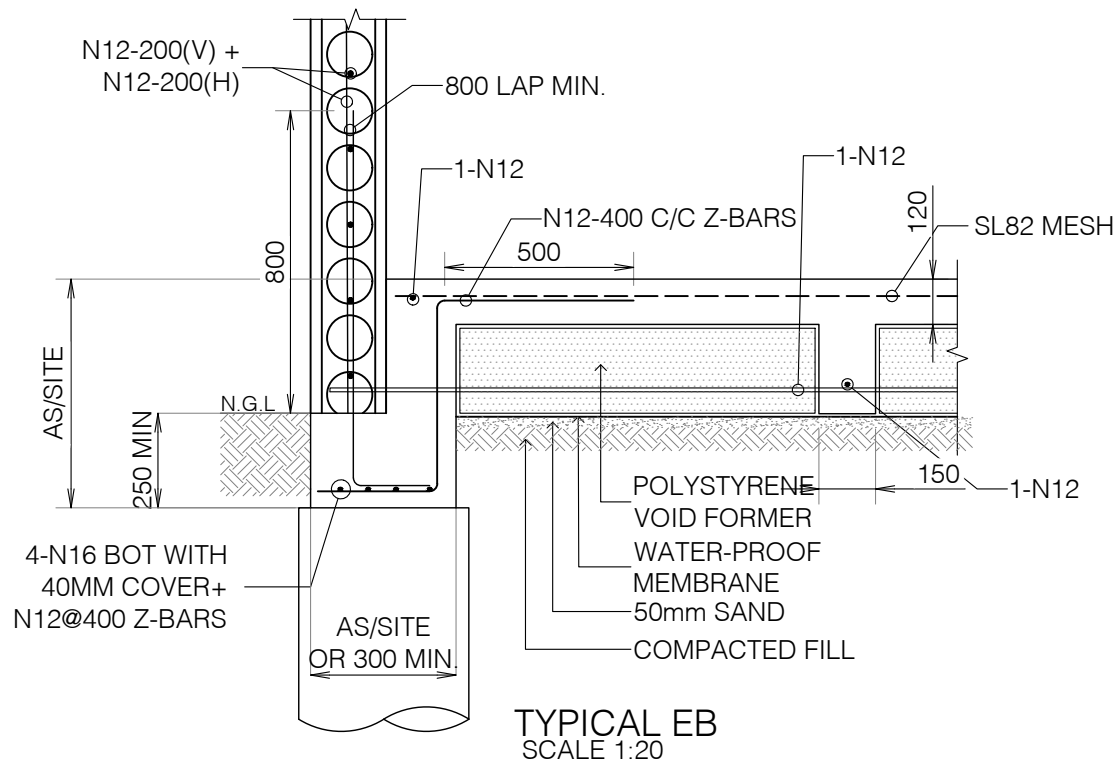
NOTES:  
ALL CONCRETE IS TO BE 32MPa  
STRESS GRADE U.N.O

NOTES:  
ARTICULATED JOINT TO BE PROVIDED AT  
EVERY 5.5M AS PER THE REGULATIONS OF  
AS3700 CLAUSE 4.8.4. FOR ANY RELAXATION  
OF MORE THAN 100MM, APPROVAL MUST BE  
OBTAINED FROM THE ENGINEER

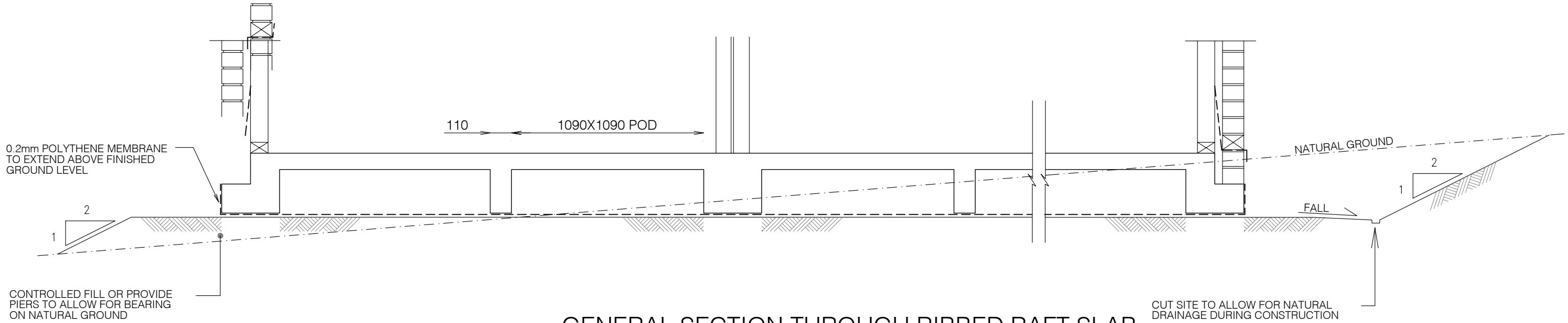
NOTES:  
ALL FILLS AND SOILS UNDER FOOTING TO  
BE COMPACTED IN LAYERS AS PER AS2870  
AND AS3798.

LEGENDS AND SYMBOLS:

- SLAB THICKNESS
- BRICK VENEER WALLS OVER
- 450 DIA MASS CONCRETE PIER REFER TO SHEET S-04
- 2000 LONG 3-N12 TM TRIMMER BARS
- STARTING POINT
- 1090x1090MM POD
- POD SIZE AS/SITE

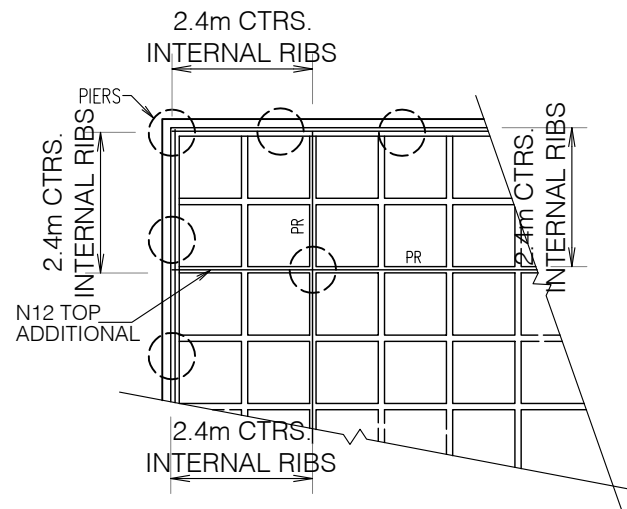


REV	DATE	DESCRIPTION
1	28/01/22	



GENERAL SECTION THROUGH RIBBED RAFT SLAB

NTS



TYPICAL PIER LAYOUT

SETOUT INFORMATION

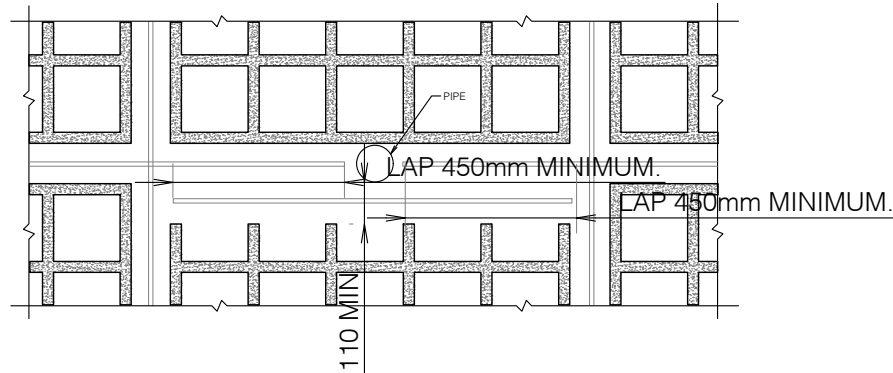
PIERS- 450 DIA.  
2.4m CENTERS EXTERNAL RIBS  
2.4M CENTERS INTERNAL RIBS

PR- PRINCIPLE RIB

PRINCIPLE INTERNAL RIB REINFORCEMENT

TOP- PROVIDE 1N12 EXTRA CONTINUOUS LAPPED 600mm AT MIDSPAN BETWEEN PIERS AD TIED TO UNDERSIDE OF MESH

BOTTOM- PROVIDE 1N12



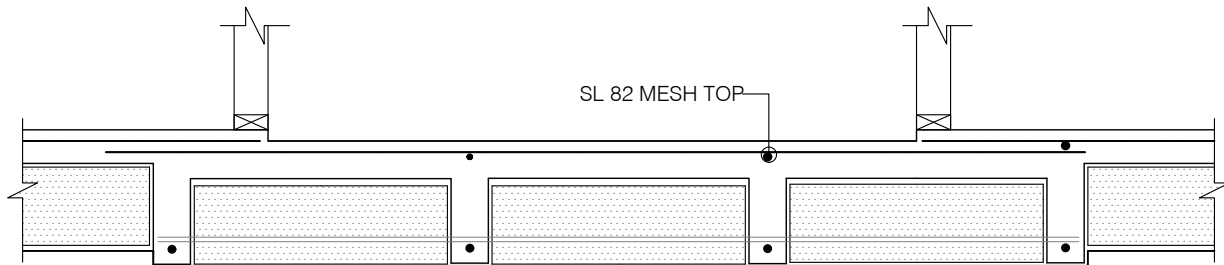
RIB PENETRATION DETAIL

NOTE: IF PLUMBING RISERS CLASH WITH RIBS MAINTAIN MINIMUM RIB WIDTH OF 110mm BY CUTTING END OFF PODS AND LAPPING REINFORCEMENT AS DETAILED.

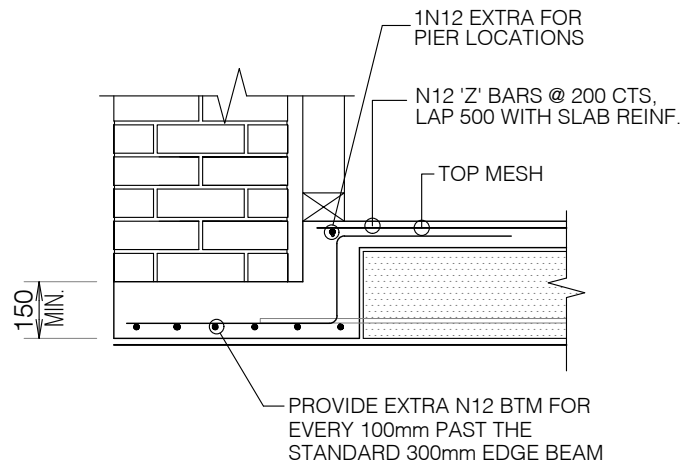
L-BAR 600 LEGS TOP AND BOTTOM

TYPICAL CORNER DETAIL

SCALE 1:20



TYPICAL TILE (30mm) SETDOWN DETAIL



LOCAL WIDENING OF EDGE BEAM

APPLICABLE UNDER ENGAGED BRICK COLUMNS, HOTWATER UNIT PADS, ETC.

REV	DATE	DESCRIPTION
1	28/01/22	

MAXIMUM UNREINFORCED PIER LENGTH	
Ø450	1.8m

450mmØ CONCRETE  
PIER BORED TO CLASS  
5 SHALE (MIN 700 KPa)

FOR REINFORCEMENT  
DETAILING CHECK TABLE  
PROVIDED FOR MAXIMUM  
UNREINFORCED LENGTH

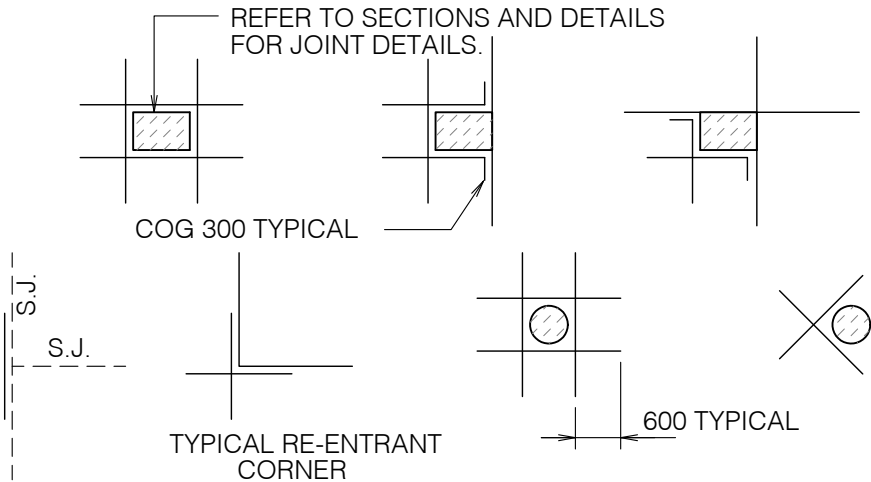
MIN. 150  
EMBEDMENT

TYPICAL PIER  
DETAIL

700 KPa CLASS 5 WEATHERED SHALE  
TO BE CONFIRMED BY GEOTECHNICAL  
ENGINEER.

TYPICAL Ø450  
PIER PLAN VIEW

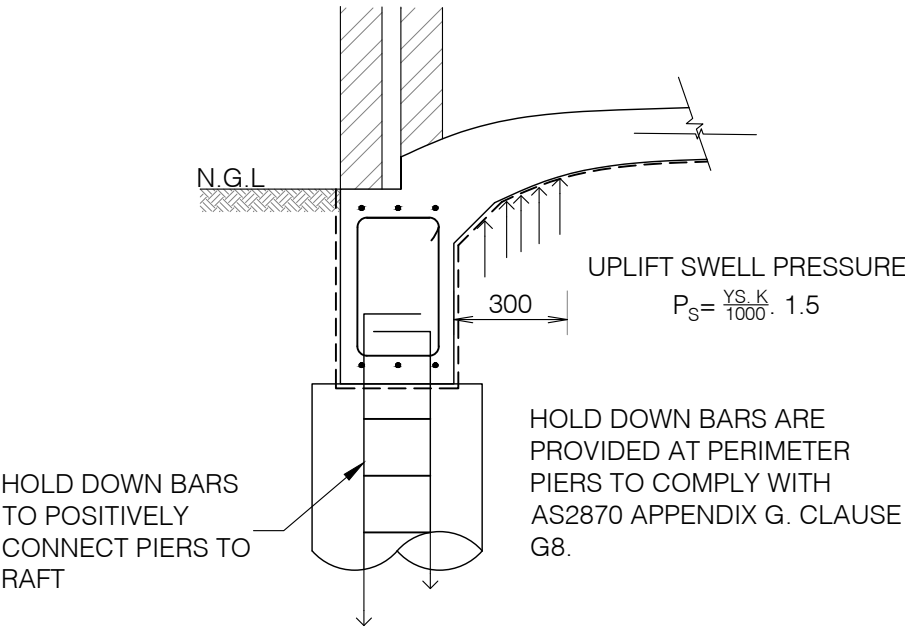
6 N16 WITH R10  
TIES @150mm  
C/C



ALL TRIMMERS TO BE 1N16 U.N.O.

## TYPICAL SLAB ON GROUND TRIMMER DETAILS

AT ALL COLUMNS, WALLS, PITS, FLOOR WASTES, ETC  
THAT CAUSE A PENETRATION THROUGH THE SLAB.



CONSULTANTS:

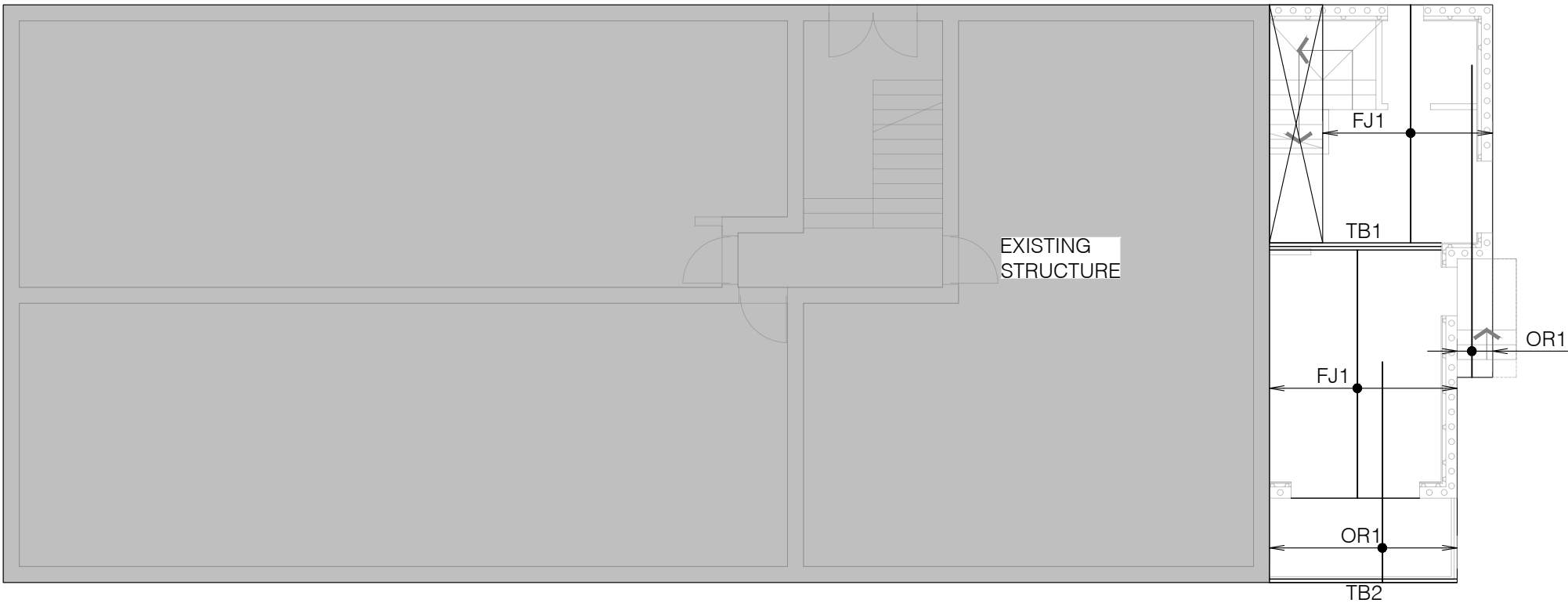
REV	DATE	DESCRIPTION
1	28/01/22	

**COUNCIL:** Canterbury-Bankstown Council  
**DRAWN BY:** A.H  
**DESIGNED BY:** R.D  
**CLIENT:** B. Adasi

**DRAWING TITLE:** Typical Pier Details

**SITE ADDRESS:** 48 Cornelia St, Wiley Park 2195  
**LOT:** - **SEC:** - **SP:** 20535  
**ISSUED FOR:** CDC  
**PROJECT:** Addition & Alteration

**SCALE:** 1:20 @A3 **DATE:** 28/01/22 **REV:** R1 **SHEET NO.:** S-04



# FIRST FLOOR BEAM LAYOUT

SCALE 1:100

NOTES:

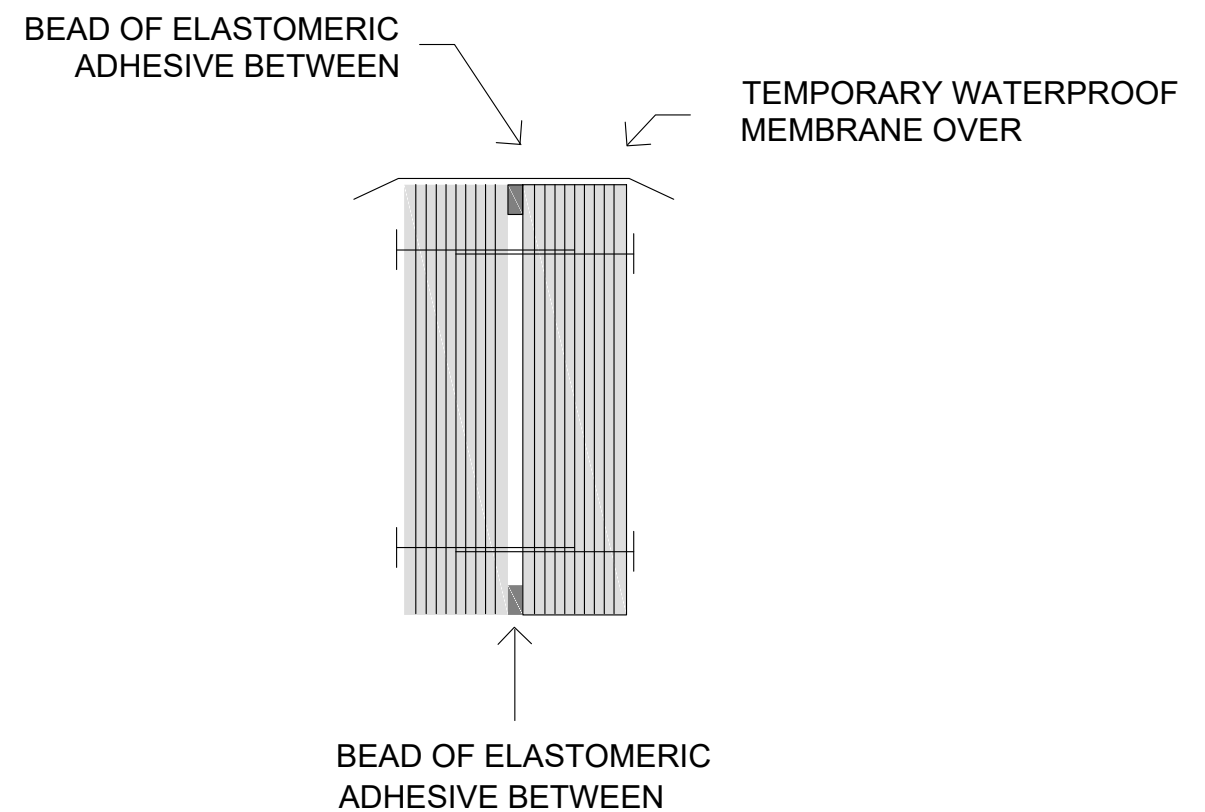
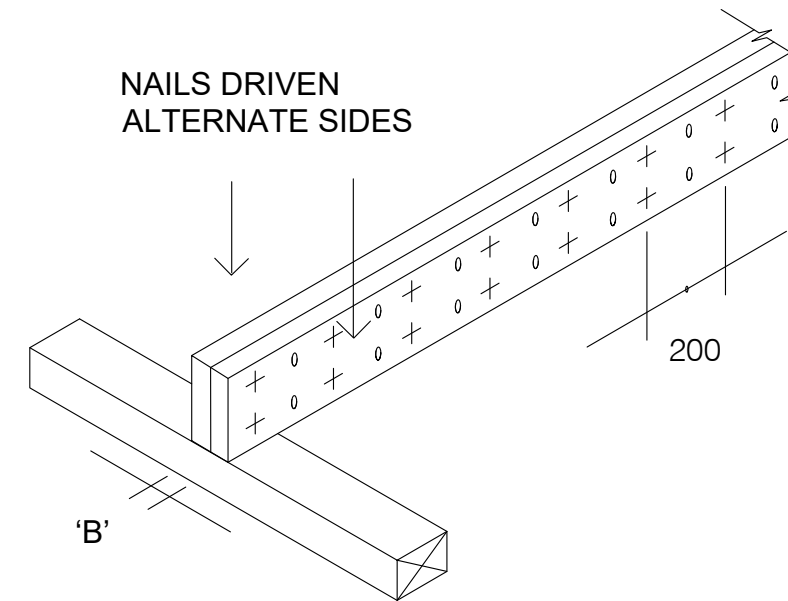
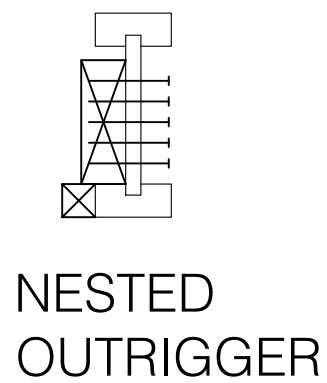
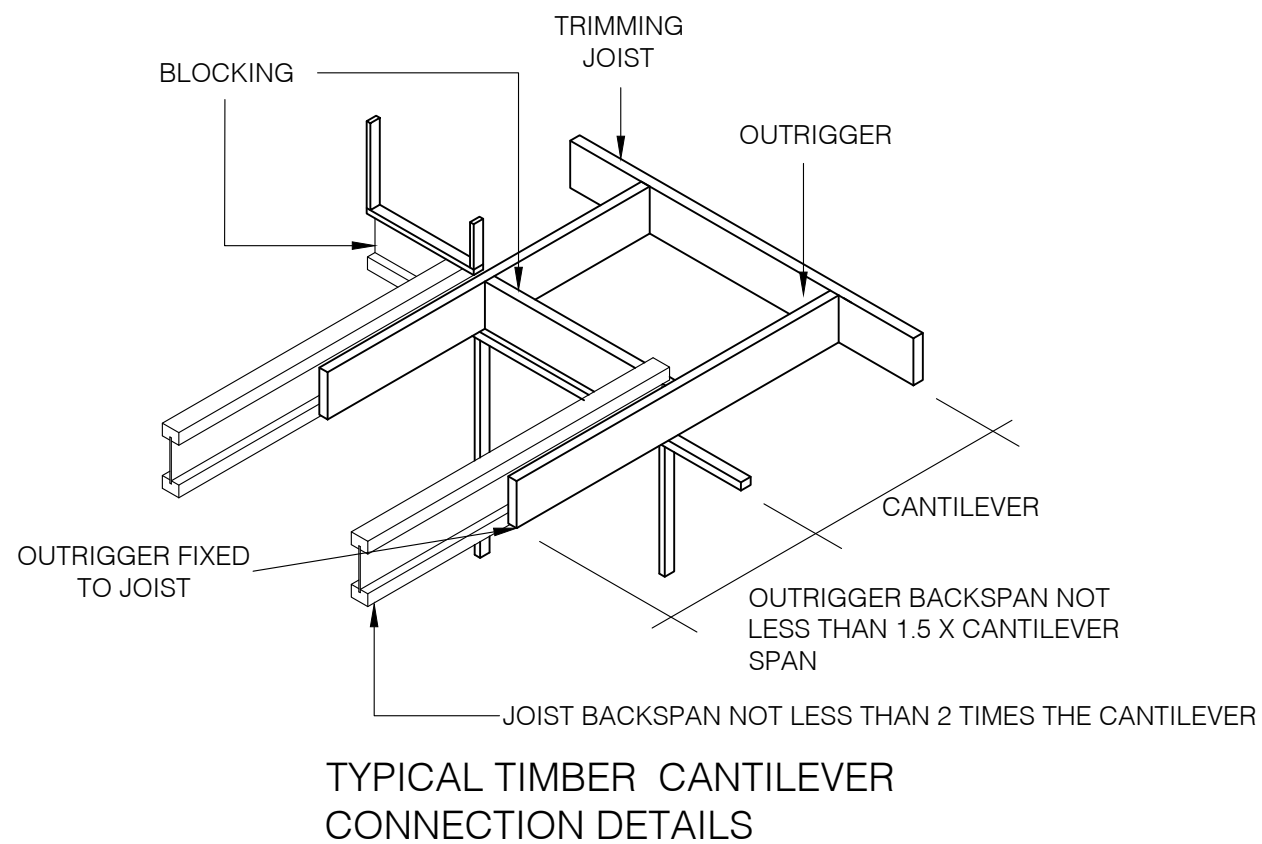
1. DRAWINGS TO BE READ IN CONJUNCTION WITH ARCHITECTURALS.
2. REFER TO ARCHITECTURAL DRAWINGS FOR ALL SETOUT, LEVELS, FALLS ETC.

## LEGENDS AND SYMBOLS:

 WALLS UNDER

BEAM AND JOIST SCHEDULE	
TB1	2/300x63 hySPAN LVL TIMBER BEARER
	VERTICAL LAMINATION TO BE PROVIDED AS PER DETAIL H1 - REFER S-06
TB2	200x45 hySPAN LVL TIMBER BEARER
FJ1	HJ300x63 @ 450 C/C hyJOIST LVL TIMBER JOISTS
OR1	200x45 hySPAN LVL OUTRIGGER

REV	DATE	DESCRIPTION
1	28/01/22	



REV	DATE	DESCRIPTION
1	28/01/22	

Lintels  
In Upper Storey Load Bearing External Walls

hySPAN SECTION D x B (mm)	SHEET ROOF AND CEILING				
	ROOF LOAD WIDTH 'RLW' (m)				
	1.8	3.0	4.2	5.4	6.6
	MAXIMUM SINGLE SPAN (m)				
90 x 45	1.9	1.5	1.3	1.1	1.0
120 x 35	2.4	2.0	1.7	1.5	1.3
120 x 45	2.6	2.2	1.9	1.7	1.5
140 x 35	2.7	2.4	2.1	1.8	1.7
140 x 45	2.9	2.5	2.4	2.1	1.9
150 x 35	2.8	2.5	2.3	2.0	1.8
150 x 45	3.0	2.7	2.4	2.3	2.0
170 x 35	3.1	2.7	2.5	2.4	2.1
170 x 45	3.3	2.9	2.7	2.5	2.4
190 x 35	3.4	3.0	2.8	2.6	2.5 <sup>(45)</sup>
190 x 45	3.6	3.2	2.9	2.8	2.6
200 x 35	3.5	3.1	2.9	2.7	2.5 <sup>(45)</sup>
200 x 45	3.7	3.3	3.0	2.8	2.7
200 x 63	4.0	3.5	3.3	3.1	2.9
240 x 35	4.1	3.6	3.3	3.1 <sup>(45)</sup>	2.9 <sup>(45)</sup>
240 x 45	4.3	3.8	3.5	3.3	3.1
240 x 63	4.6	4.1	3.7	3.5	3.4
290 x 35	4.8	4.1	3.6	3.2	2.8 <sup>(45)</sup>
290 x 45	5.0	4.4	4.0	3.8	3.6 <sup>(45)</sup>
300 x 45	5.0	4.5	4.1	3.8	3.7 <sup>(45)</sup>
300 x 63	5.4	4.8	4.5	4.2	4.0
360 x 63	6.1	5.5	5.1	4.8	4.6

Lintels  
In Lower Storey Load Bearing External Walls

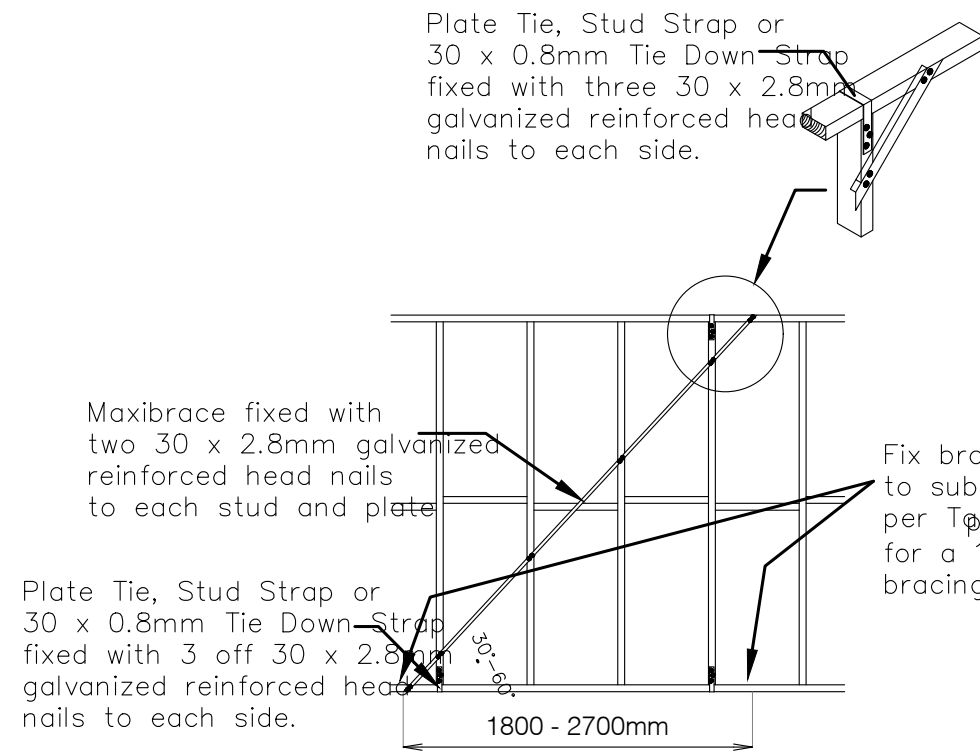
hySPAN SECTION D x B (mm)	SHEET ROOF AND CEILING								
	FLOOR LOAD WIDTH 'FLW' (m)								
	1.8			2.4			3.0		
	ROOF LOAD WIDTH 'RLW' (m)								
	1.8	4.2	6.6	1.8	4.2	6.6	1.8	4.2	6.6
	MAXIMUM SINGLE SPAN (m)								
130 x 35	1.6	1.4	1.3	1.5	1.4	1.3	1.4	1.3	1.2
140 x 45	1.9	1.7	1.6	1.8	1.6	1.5	1.7	1.6	1.5
150 x 45	2	1.8	1.7	1.9	1.7	1.6	1.8	1.6	1.5
170 x 45	2.2	2.0	1.9	2.1	1.9	1.8	2.0	1.8	1.7
190 x 45	2.5	2.3	2.1	2.4	2.2	2.1	2.2	2.1 <sup>(45)</sup>	2.0 <sup>(45)</sup>
200 x 45	2.6	2.4	2.2	2.5	2.3	2.1	2.3 <sup>(45)</sup>	2.2 <sup>(45)</sup>	2.0 <sup>(45)</sup>
200 x 63	2.9	2.6	2.5	2.7	2.5	2.4	2.6	2.4	2.3
2/200 x 45*	3.2	3.0	2.8	3.1	2.8	2.7	2.9	2.7	2.6
240 x 45	3.1	2.9	2.7 <sup>(45)</sup>	3.0 <sup>(45)</sup>	2.8 <sup>(45)</sup>	2.6 <sup>(45)</sup>	2.8 <sup>(45)</sup>	2.6 <sup>(70)</sup>	2.5 <sup>(70)</sup>
240 x 63	3.4	3.1	2.9	3.2	3.0	2.8	3.1	2.9 <sup>(45)</sup>	2.7 <sup>(45)</sup>
2/240 x 45*	3.7	3.5	3.3	3.5	3.3	3.2	3.4	3.2	3.1
290 x 45	3.6 <sup>(45)</sup>	3.4 <sup>(45)</sup>	3.2 <sup>(45)</sup>	3.4 <sup>(45)</sup>	3.2 <sup>(70)</sup>	3.1 <sup>(70)</sup>	3.3 <sup>(70)</sup>	3.1 <sup>(70)</sup>	3.0 <sup>(70)</sup>
300 x 45	3.6 <sup>(45)</sup>	3.4 <sup>(45)</sup>	3.2 <sup>(45)</sup>	3.5 <sup>(45)</sup>	3.3 <sup>(70)</sup>	3.1 <sup>(70)</sup>	3.3 <sup>(70)</sup>	3.2 <sup>(70)</sup>	3.0 <sup>(70)</sup>
300 x 63	4.0	3.7	3.5	3.8	3.6 <sup>(45)</sup>	3.4 <sup>(45)</sup>	3.6 <sup>(45)</sup>	3.4 <sup>(45)</sup>	3.3 <sup>(70)</sup>
2/300 x 45*	4.3	4.0	3.8	4.1	3.9	3.7	4.0	3.8	3.6 <sup>(45)</sup>
360 x 63	4.5	4.2 <sup>(45)</sup>	4.0 <sup>(45)</sup>	4.3 <sup>(45)</sup>	4.1 <sup>(45)</sup>	3.9 <sup>(70)</sup>	4.2 <sup>(70)</sup>	3.9 <sup>(70)</sup>	3.8 <sup>(70)</sup>

TYPICAL LINTEL SCHEDULE

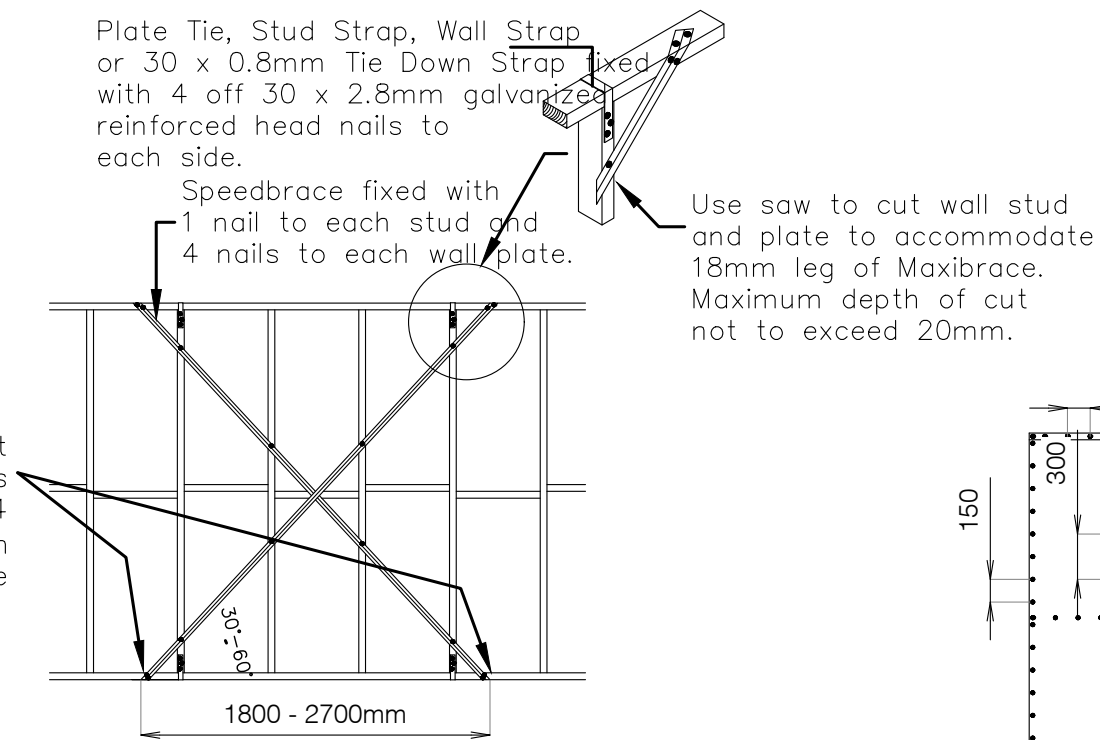
- SCALE 1:100
1. MINIMUM BEARING 35 MM AT END SUPPORTS UNLESS NOTED OTHERWISE AS SPAN(BEARING).

REV	DATE	DESCRIPTION
1	28/01/22	

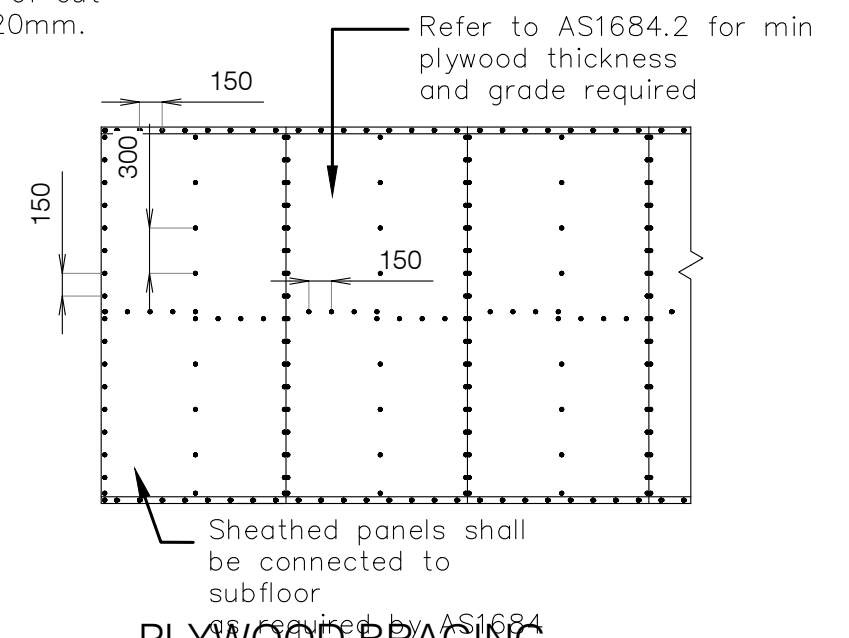




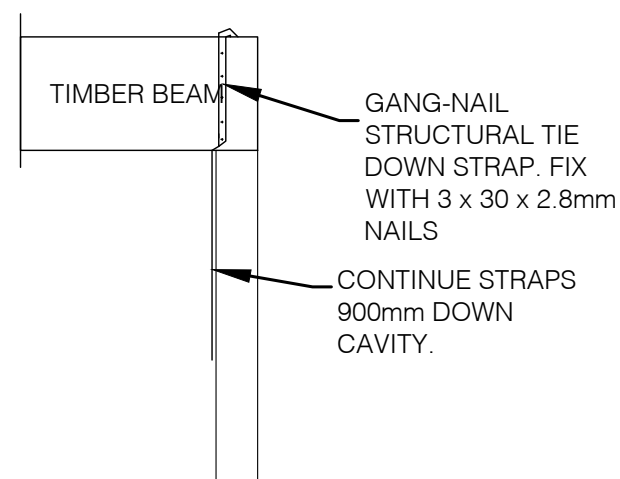
**MAXI BRACE**  
1.5kN/m BRACING TYPE



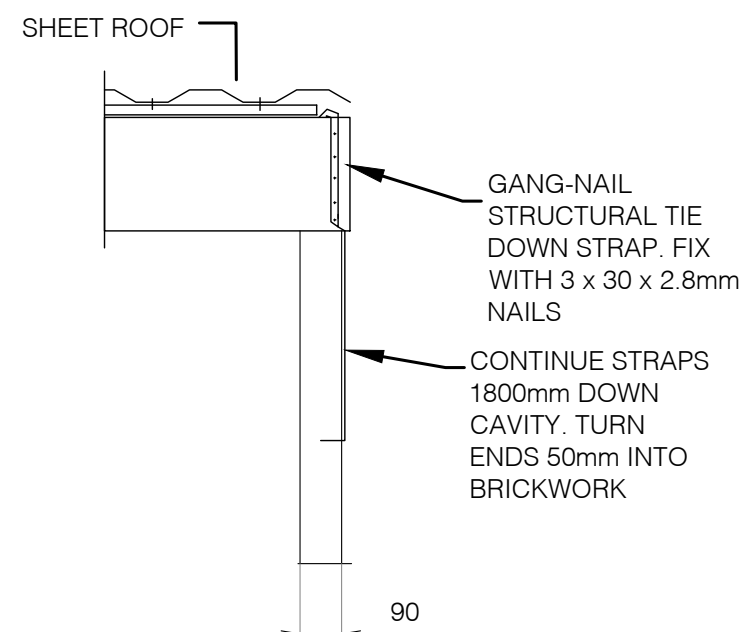
**MAXI-SPEEDBRACE**  
3.0kN/m BRACING TYPE



**PLYWOOD BRACING**  
3.4kN/m BRACING TYPE

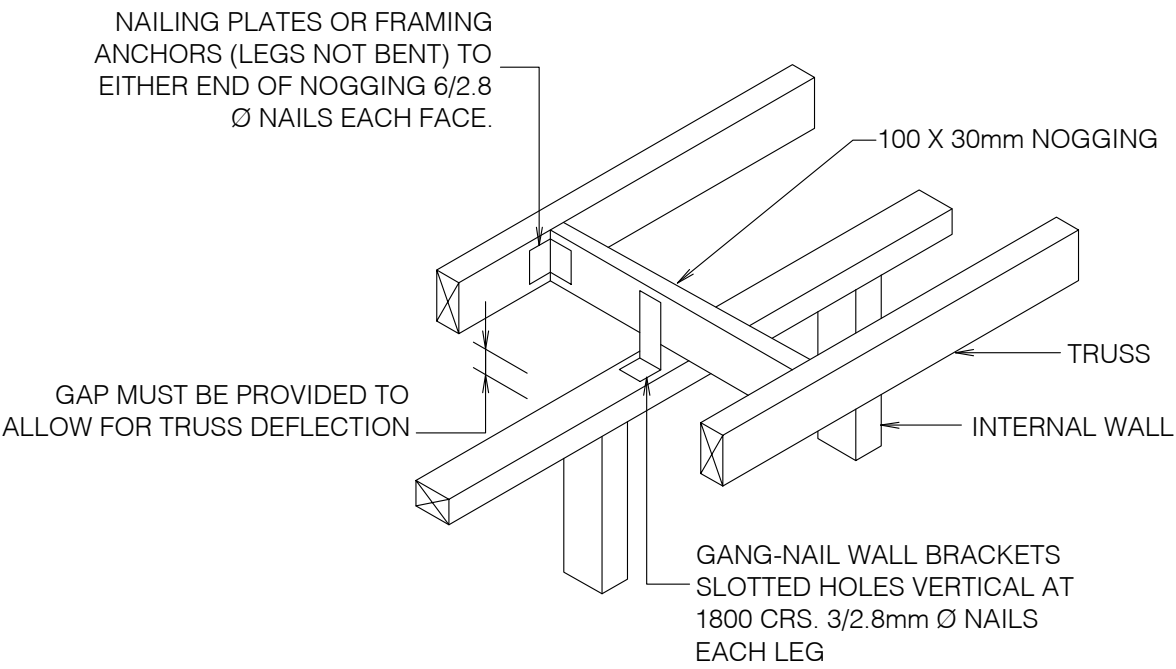


**STRUCTURAL TIE DOWN DETAILS**

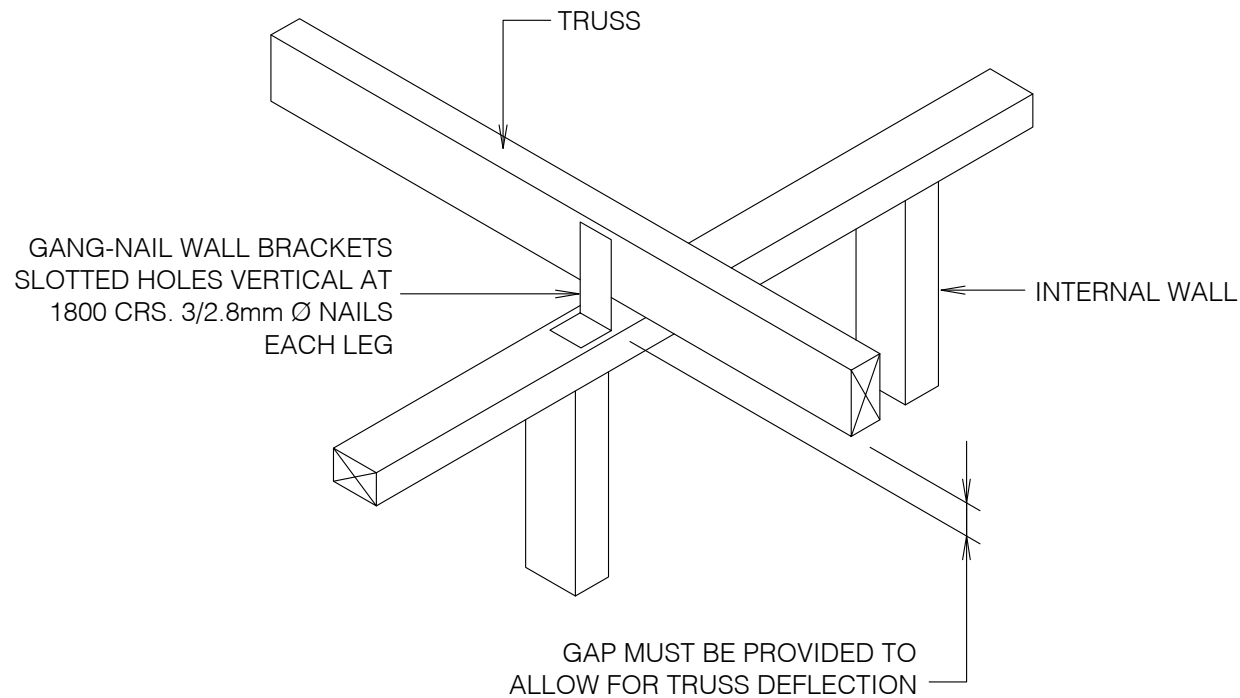


**DETAIL B**  
(STRUCTURAL TIE DOWN DETAILS)

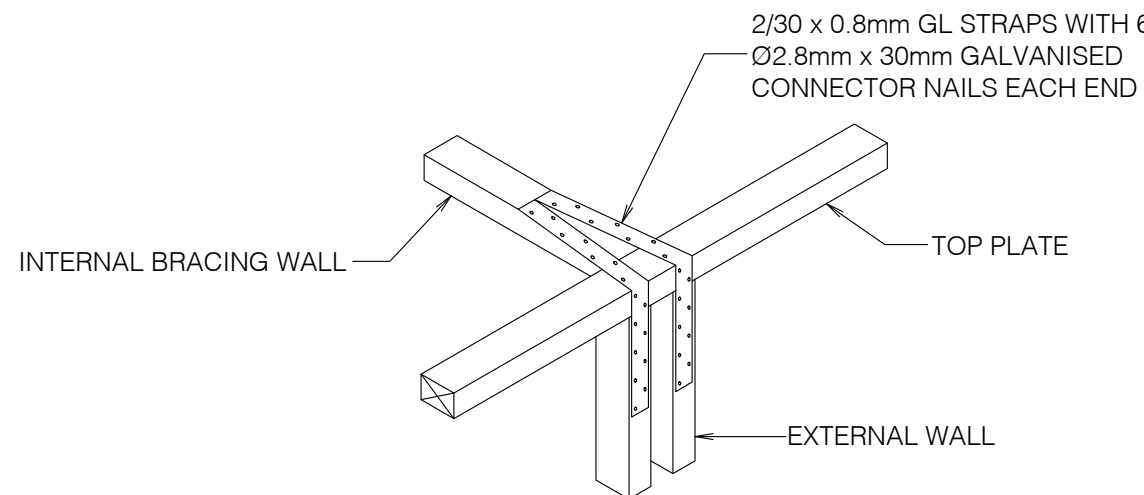
REV	DATE	DESCRIPTION
1	28/01/22	



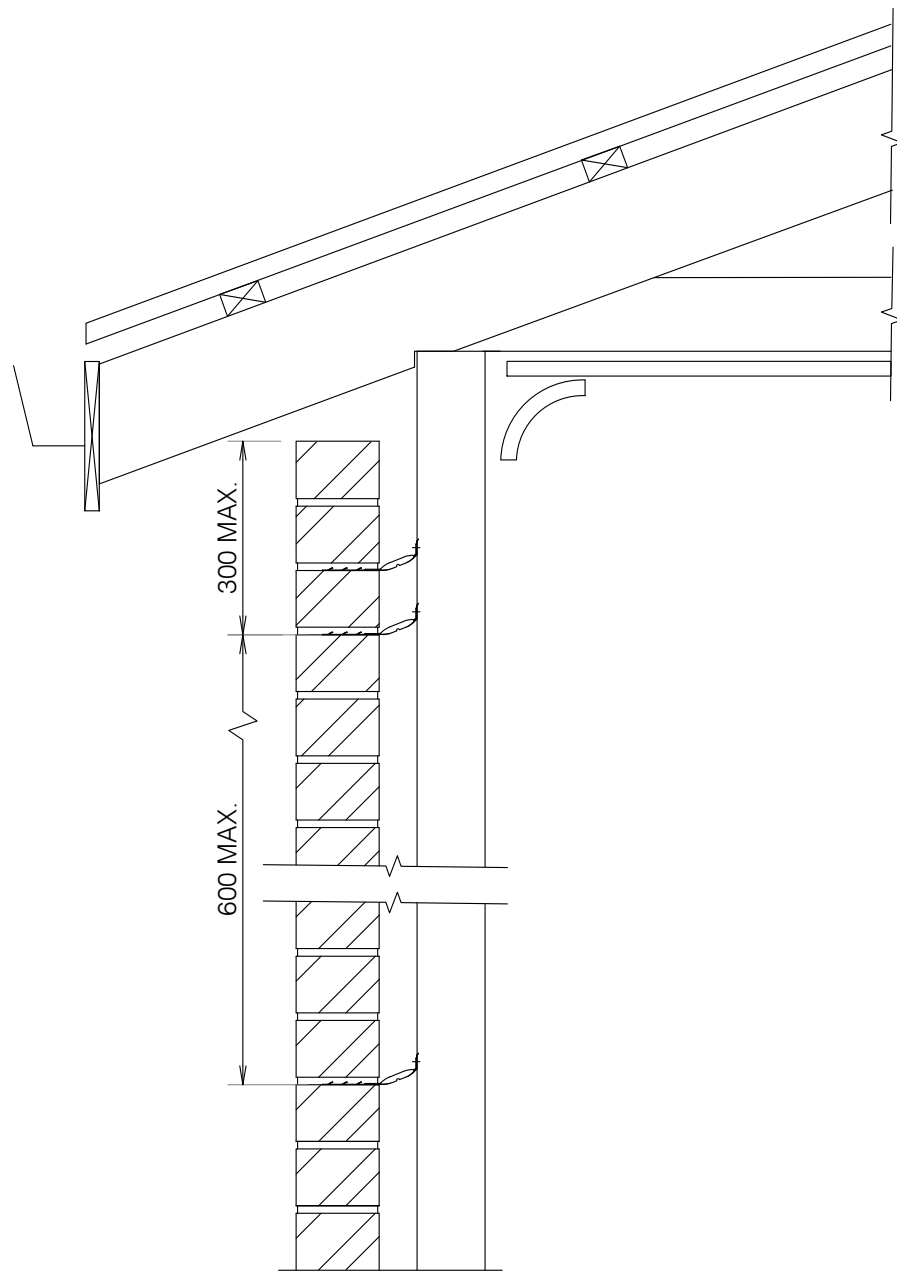
NON LOAD BEARING WALL PARALLEL TO TRUSS



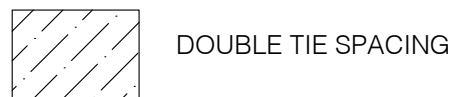
NON LOAD BEARING WALL PERPENDICULAR TO TRUSS



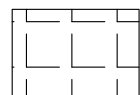
INTERNAL BRACING WALL TO EXTERNAL WALL CONNECTION



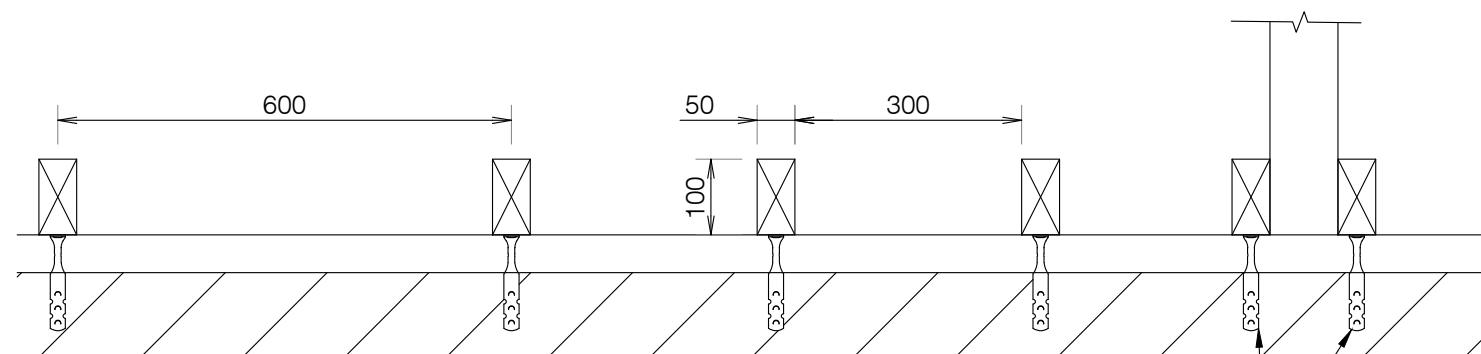
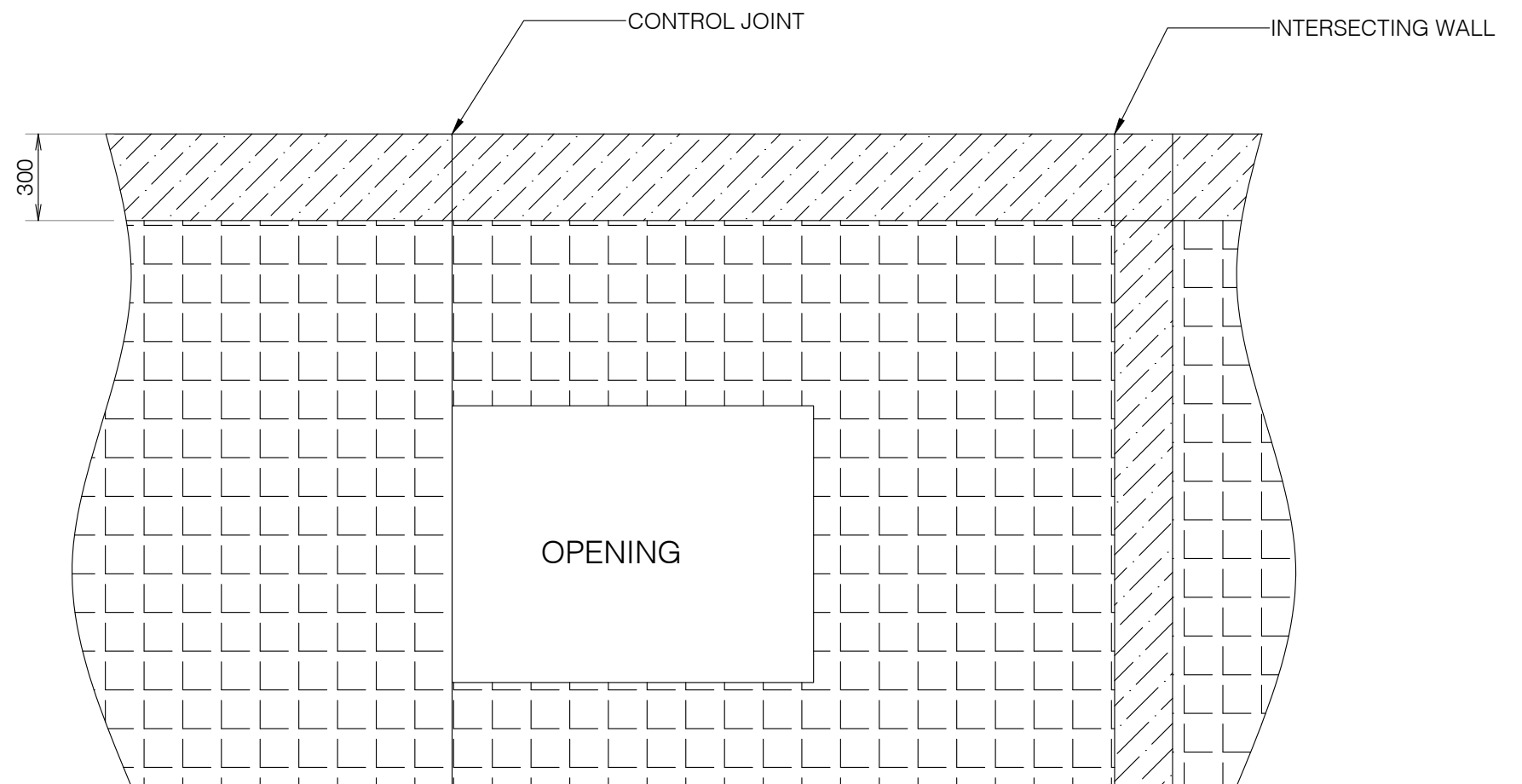
#### LEGEND:



DOUBLE TIE SPACING



MEDIUM DUTY TIES @ 600 C/C  
MAX.



AT INTERSECTING WALL  
DOUBLE THE NUMBER OF  
TIES

#### VENEER TIES DISTRIBUTION

#### MEDIUM DUTY TIES

#### CONSULTANTS:

REV	DATE	DESCRIPTION
1	28/01/22	

#### COUNCIL:

Canterbury-Bankstown Council

#### DRAWN BY:

A.H

#### DESIGNED BY:

R.D

#### CLIENT:

B. Adasi

#### DRAWING TITLE:

Typical Wall Connection Details

**SITE ADDRESS:** 48 Cornelia St, Wiley Park 2195

**LOT:** - **SEC:** - **SP:** 20535

**ISSUED FOR:** CDC

**PROJECT:** Addition & Alteration

**SCALE:** NTS

**DATE:** 28/01/22

**REV:** R1

**SHEET NO.:** S-10

PROJECT NO. 22-004